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Scour Stability Evaluation of Bridge Pier Considering Fluid-Solid Interaction

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Outline

- Introduction
- Methodology
- Methodology Verification
- Practical Application
- Summary and Conclusion



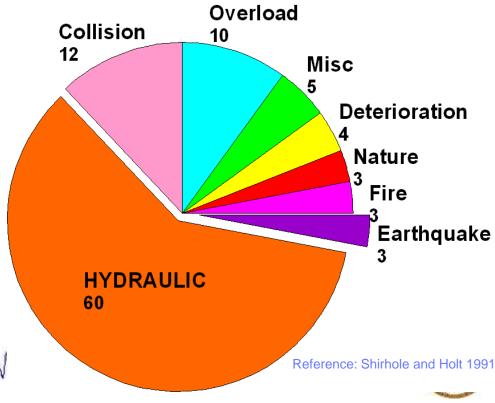
I-5 Skagit River Bridge in the state of Washington (2013)



The Nanfang'ao Bridge collapsed in Taiwan (2019)



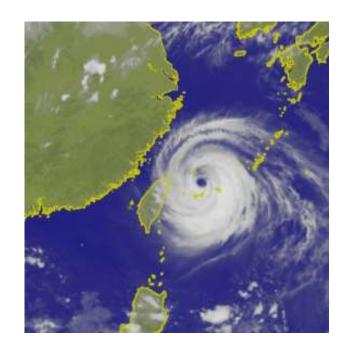
- Scour is one of the major causes for bridge failure.
- More than 1000 bridges have collapsed over the past 30 years in the U.S., with 60% of the failures due to scour.





Scour at bridge

- Typhoon and hurricane can bring intensive rainfall in short term of time.
- flash flood or other flood hazard may occur in catchment area with bad soil and water conservation.
- Scouring and sedimentation happen everywhere at riverbed.
- When water flows by obstacle, such as pier and abutment, scouring may have significant impact on bridge stability.

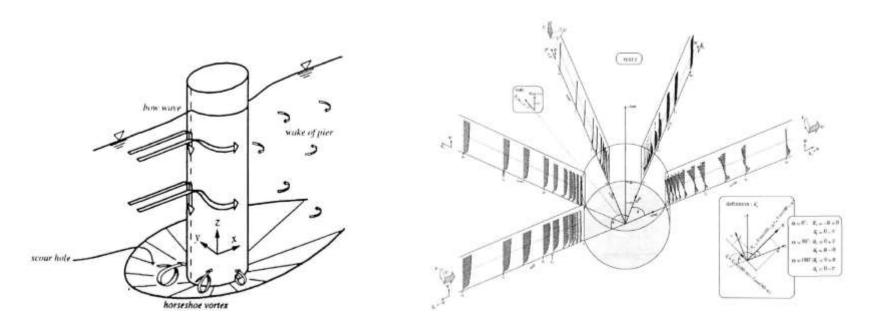






Fluid behavior at pier

- Flow pattern and vortex around cylinder or scoured pier have been descripted by Graf and Altinakar(1998), Graf W.H. and B. Yulistiyanto(1998) and other researchers.
- Fluid behavior associated to scouring can be concluded into 3 types: downward flow, horseshoe vortex and wake-vortex system



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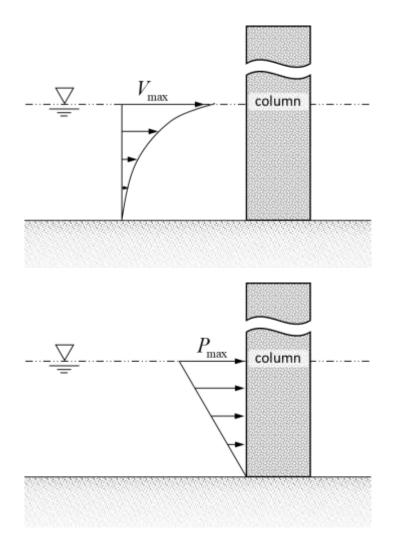


Current regulations

- However, most current regulations simplified the complicated fluid behavior into parabolic current speed and linear current force to conduct static evaluation.
- The negligence of nonlinear force generated by fluid flow might cause overestimation of pier stability.

$$P_{avg} = 52.5K \left(V_{avg}\right)^2$$

$$P_{\text{max}} = 2P_{avg}$$





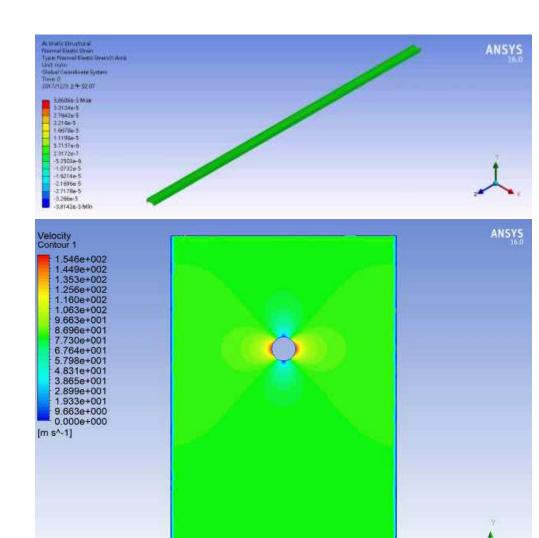
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Numerical simulation

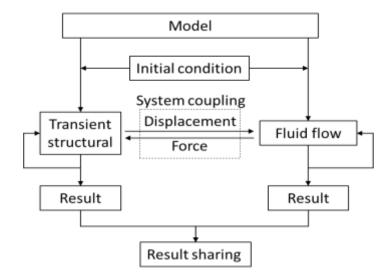
- For most finite element simulation program, solid and fluid simulation work separately, meaning fluid and solid condition can not be considered simultaneously.
- Finite element simulation program ANSYS is applied to conduct static and dynamic fluid-solid interaction (FSI) simulation.

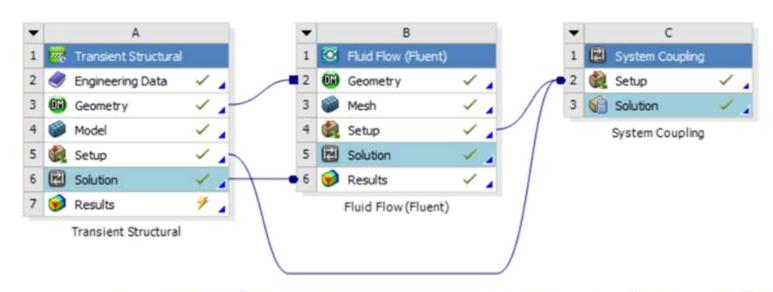




Fluid-Solid Interaction

■ By using system coupling, result from both solid and fluid simulation system is transferred to each other as the boundary condition of next time interval.

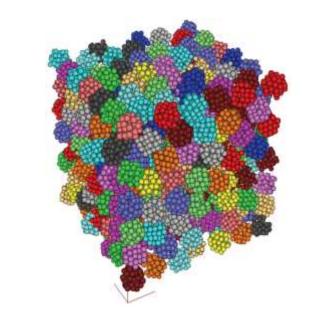


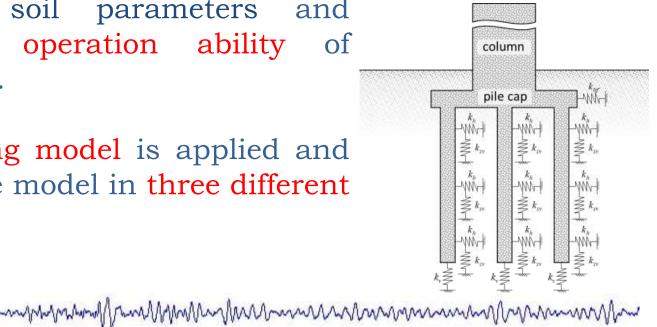




Soil model

- In order to simulate soil behavior, several methods such as solid elements, discrete element method (DEM) and soil spring are applied.
- Solid elements and DEM required detailed soil parameters and powerful operation ability of hardware.
- Soil spring model is applied and set on the model in three different types.







Soil spring

■ Referring to *Seismic Design Specification* of Taiwan, the soil spring model includes four different types of spring:

Horizontal pile soil spring

$$k_h = 0.34 (\alpha E_0)^{1.10} D^{-0.31} (EI)^{-0.103}$$

Horizontal pile cap soil spring

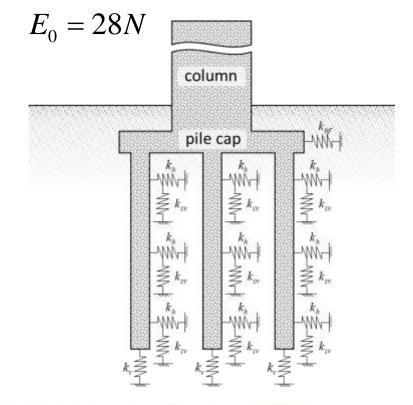
$$k_{hf} = k_{h0} \left(\frac{B_H}{30}\right)^{-3/4} \qquad k_{h0} = \frac{(\alpha E_0)}{30}$$

Vertical pile toe soil spring

$$k_{v} = k_{v0} \left(\frac{B_{v}}{30}\right)^{-3/4}$$
 $k_{v0} = \frac{(\alpha E_{0})}{30}$

Vertical pile soil spring

$$k_{sv} = 0.3k_h$$





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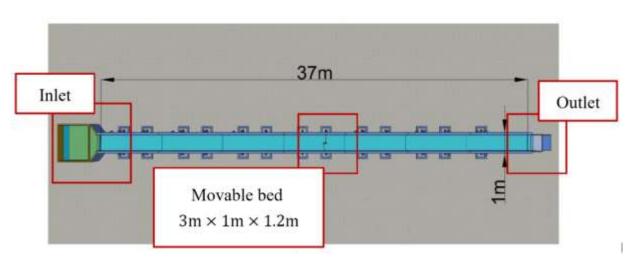


Scaled scour experiment

- As only few studies applying FSI system to scour pier simulation, it is necessary to verify the applicability of FSI system.
- A scaled scour test with 12 cm initial buried depth and 2.5 m/s flow velocity was conducted for verification.

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■ Velocity meters are set on top of pier model to collect the vibration in 3 directions.

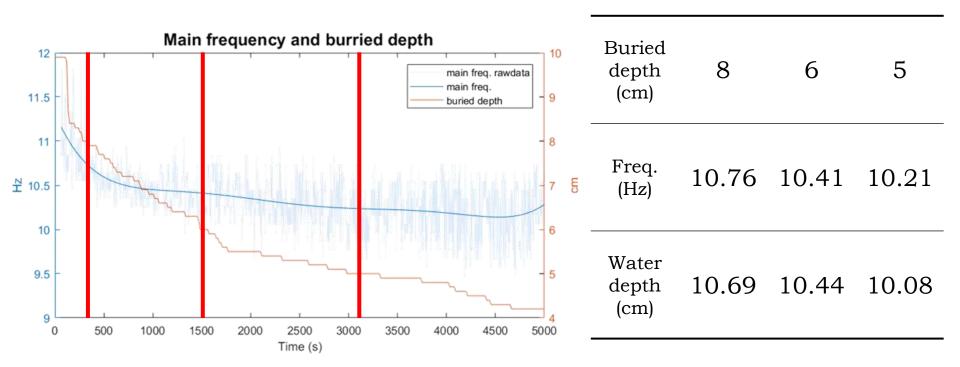






Experimental analysis

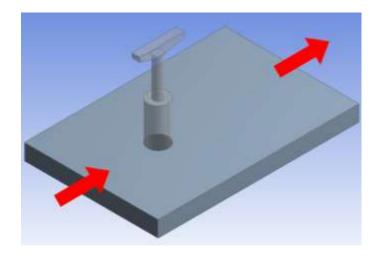
- By using STFT, main frequency of pier model at each time in the current direction is identified.
- 3 boundary conditions (8, 6, 5 cm) are selected for numerical simulation

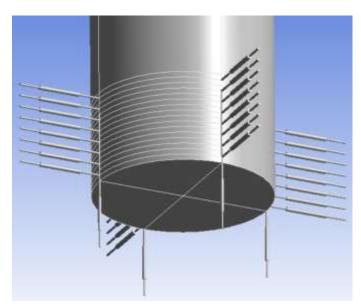




Numerical model

- Pier model is surrounded by a 100 x100 cm fluid block. The current speed is increased gradually to 2.5m/s.
- Springs are set in current, vehicle and vertical direction. To prevent lack of stability, each soil spring is separated to several parts and set evenly on the model.



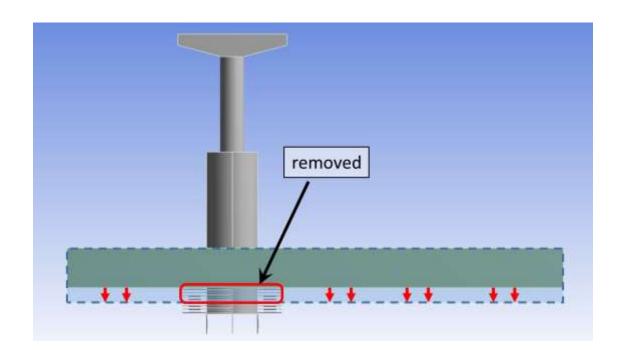


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Fluid block setting

- The boundary condition is unchangeable during FSI simulation.
- Scour simulation is conducted by adjusting the depth of fluid block and setting of soil spring.

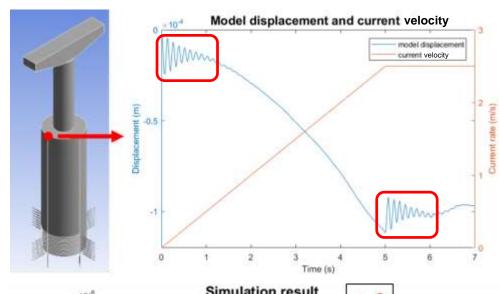


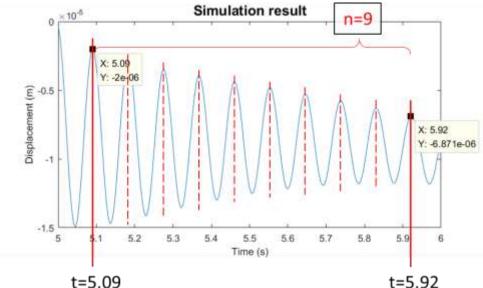


Simulation result

Due to the setting of flow velocity, two free decay signals with same frequency appear at start of simulation and steady flow velocity part.

Structural response at steady flow velocity is collected to calculate the dominant frequency of the model.





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Simulation result (Cont'd)

- While having 25% (2 cm scour) and 37.5% (3 cm scour) bed material loss, numerical model has only 3.2% and 5% error on fundamental frequency.
- Numerical model fits the test model well while the boundary condition of embedded depth is changed.

Embedded depth (cm)	8	6	5
Freq. (experiment)	10.76	10.41	10.21
Freq. (simulation)	10.77	10.07	9.70
Error	0.1%	3.2%	5.0%



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Practical structure

- Located in western
 Taiwan, Xi-Bin bridge is
 one of the most important
 bridges of western traffic
 line.
- Connecting ChangHua and YunLin county, Xi-Bin bridge crosses the longest river in Taiwan, Chou-shui river.

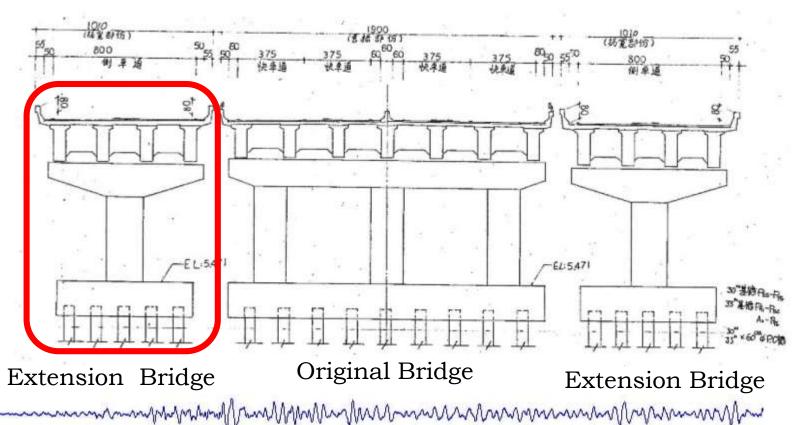






Practical structure

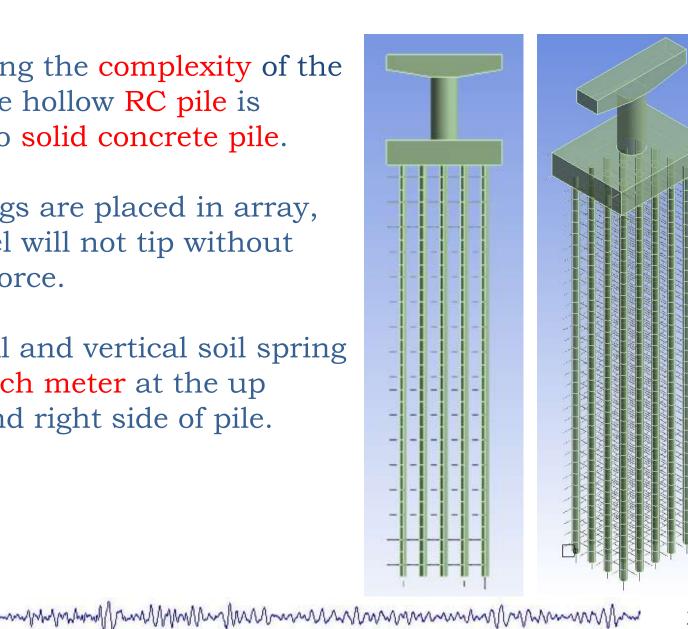
- The bridge includes a 4-lane original bridge and two 2-lane extension bridges.
- The 16th pier at the extension bridge is built in FSI system and conducted with both static and FSI simulations.





Numerical model

- Considering the complexity of the model, the hollow RC pile is simplify to solid concrete pile.
- Soil springs are placed in array, and model will not tip without external force.
- Horizontal and vertical soil spring are set each meter at the up stream and right side of pile.

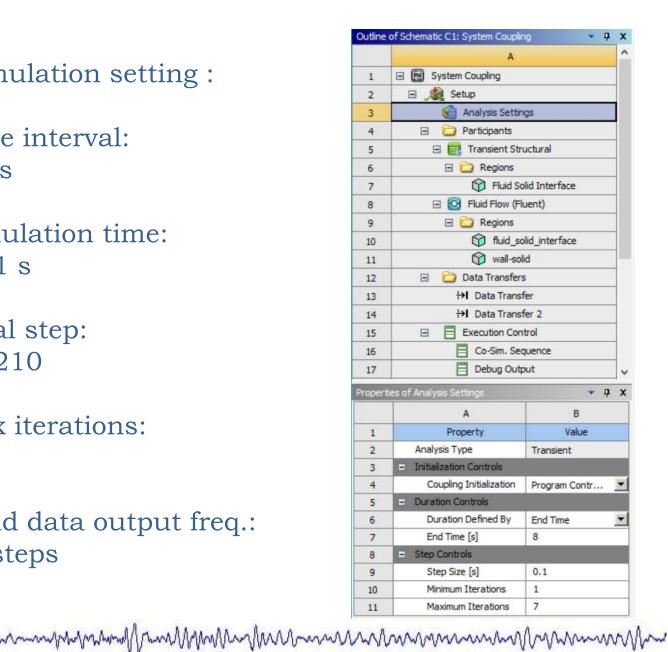




FSI simulation setting

FSI simulation setting:

- Time interval: $0.1 \, s$
- Simulation time: 8-21 s
- Total step: 80-210
- Max iterations:
- Fluid data output freq.: 50 steps





Sensitivity analysis

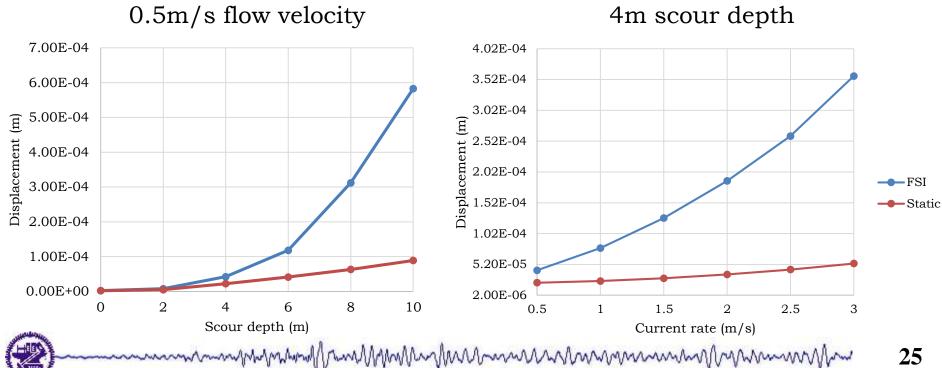
- Three boundary conditions including scour depth, flow velocity and water level are considered.
- Sensitivity analysis is conducted by comparing the response under 10m scour, 0.5m/s flow velocity and water level at EL.8.38m (3m above pile cap).
- Scour depth and flow velocity are found out to control response of model.

Scour depth 10		8	6	4	2	0		
Disp.	5.83E-04	3.12E-04	1.18E-04	4.24E-05	7.94E-06	2.30E-06		
%	% -		79.73	79.73 92.73		99.60		
Flow velocity 0.5		1	1.5	2				
Disp.	Disp. 5.83E-04		1.88E-03	2.92E-03				
%	-	116.35	222.40	401.72				
Water level	3	5	1					
Disp.	Disp. 55.83E-04 5		5.28E-04					
%	-	4.75	9.47		92			
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Comparison

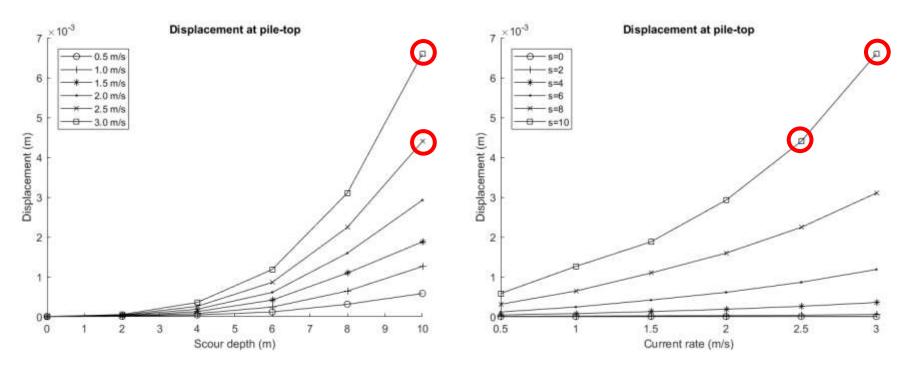
- Simulation result of the case with 4m scour and 0.5m/s flow velocity is selected for comparison
- It shows that the dynamic response is larger while considering fluid behavior, under the same scour depth or current speed.





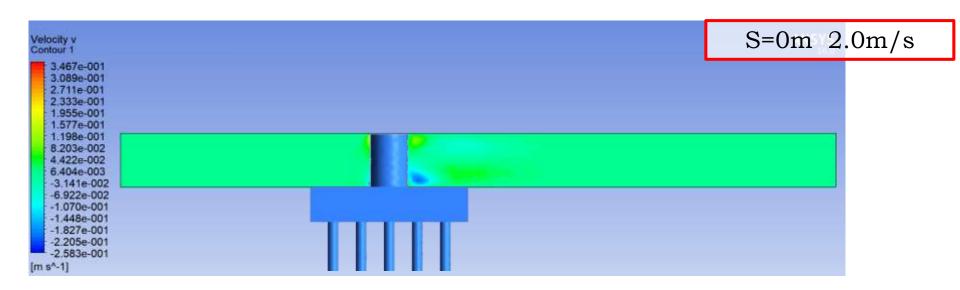
Simulation result

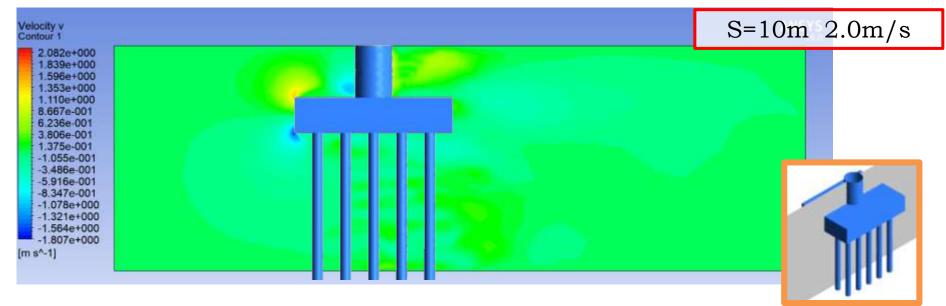
- 70 simulation cases are conducted with static and FSI system with 6 different flow velocity and scour depth condition.
- Structural response at pile top is collected.





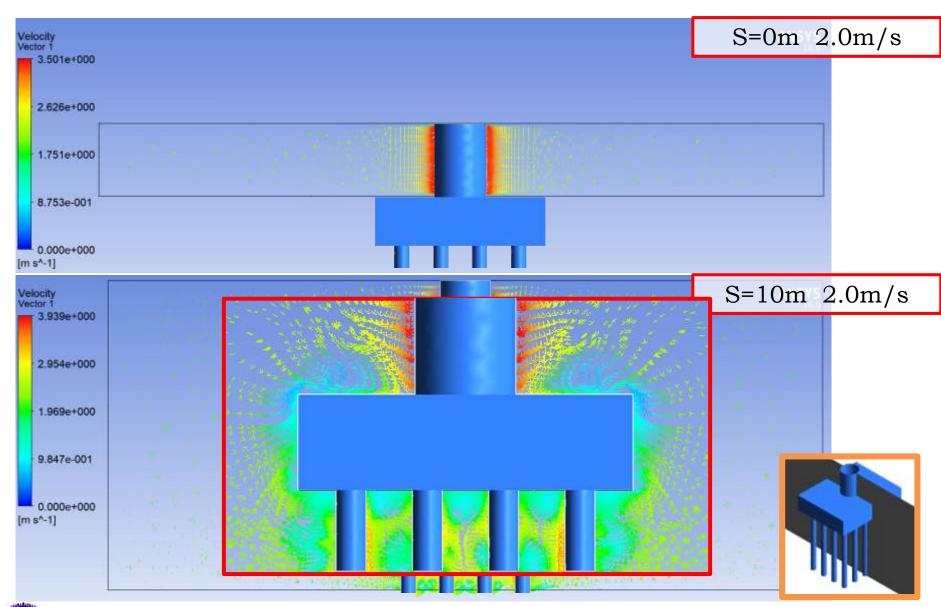
Dynamic Comparison (Front)







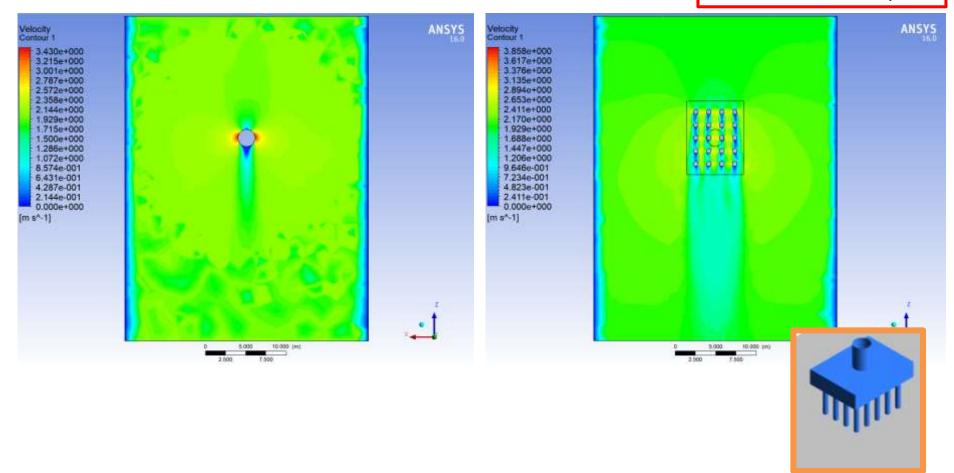
Dynamic Comparison (Tail)





Comparison (Top and Bottom)

S=10m 2.0m/s





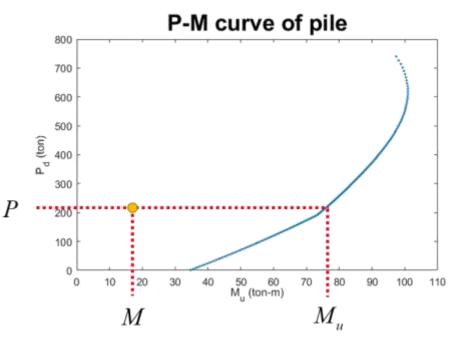
Safety factor

■ Ultimate moment (M_u) is found from the P-M curve by the actual axial force collected from the model.

$$FS = \frac{M_u}{M}$$

Allowable displacement (Δ_0) is defined by ultimate moment P and axial force.

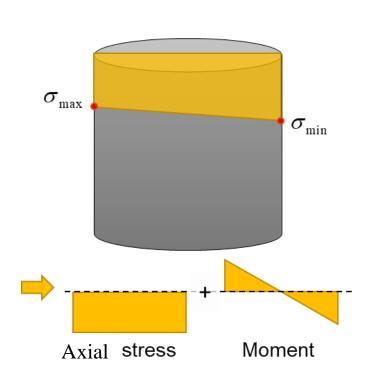
$$FS = \frac{\Delta_0}{\Lambda} \qquad \Delta_0 = 0.2 \frac{M_u}{P}$$

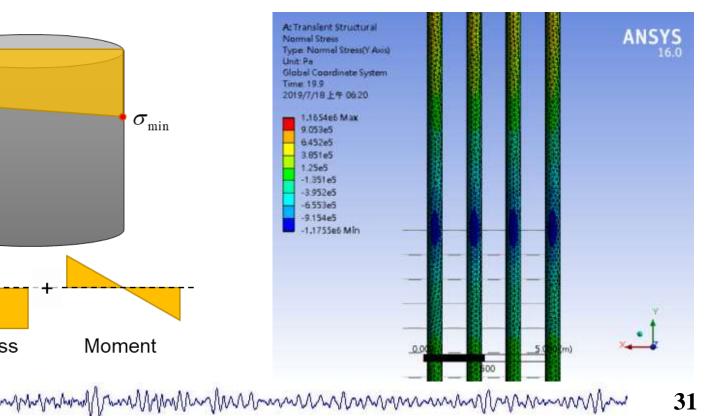




Safety factor (Cont'd)

- FSI simulation in ANSYS program provides displacement, strain and stress data of element.
- Axial stress and moment at pile is defined by stress data at steady state.







Safety factor (Cont'd)

■ Scour depth with SF over 3.0 and 2.0 are defined as alert and action level through extrapolation method.

SF of ultimate moment

SF of allowable displacement

	0.5	1	1.5	2	2.5	3		0.5	1	1.5	2	2.5	3
0	-	-	-	-	-		0	-	-	-	-	-	_
2	-	-	-	-	-		2	-	-	-	-	-	
4	927.91	1109.84	418.27	240.74	166.56	116.02	4	5546.53	2899.62	1702.79	1163.74	845.98	615.63
6	525.14	288.71	141.33	85.78	56.14	39.07	6	1542.06	758.90	458.49	308.06	215.92	156.74
8	350.64	111.06	65.45	41.25	25.95	18.05	8	621.83	293.18	180.73	119.97	81.94	59.38
10	256.33	52.93	36.02	23.38	14.27	9.92	10	307.42	140.20	87.79	57.73	38.65	27.97
12	198.44	28.89	22.11	14.70	8.75	6.08	12	172.88	76.73	48.66	31.76	20.91	15.12
14	159.82	17.31	14.64	9.93	5.79	4.02	14	106.27	46.09	29.55	19.16	12.44	8.99
16	132.50	11.11	10.24	7.07	4.04	2.81	16	69.71	29.64	19.18	12.37	7.94	5.73
18	112.30	7.52	7.47	5.24	2.95	2.05	18	48.07	20.08	13.10	8.41	5.34	3.85
20	96.86	5.30	5.64	4.01	2.22	1.54	20	34.46	14.18	9.32	5.95	3.74	2.70
22	84.73	3.86	4.37	3.14	1.72	1.20	22	25.51	10.34	6.84	4.35	2.72	1.96
24	74.99	2.89	3.46	2.52	1.36	0.95	24	19.38	7.76	5.16	3.27	2.03	1.46
26	67.02	2.22	2.79	2.05	1.10	0.76	26	15.05	5.95	3.99	2.52	1.55	1.11
28	60.39	1.73	2.29	1.70	0.90	0.63	28	11.91	4.66	3.14	1.98	1.21	0.87
30	54.82	1.38	1.90	1.43	0.75	0.52	30	9.58	3.71	2.51	1.58	0.96	0.69



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Summary and Conclusion

- Fluid-solid interaction simulation was implemented by ANSYS on a scaled pier test simulation, and is applied to practical bridge.
- Among three boundary conditions, scour depth and flow velocity influence structure displacement more then water level.
- Structure displacement at pile top is significantly larger when fluid dynamic impact is considered.
- Two safety factors are proposed to support the alert and action level for bridge health monitoring.





Thanks for your attention!